1. BUILDING CODES

A. ALL CONSTRUCTION SHALL CONFORM WITH THE 2017 (6th EDITION) FLORIDA BUILDING CODE.

B. IN ADDITION, ALL CONSTRUCTION SHALL CONFORM WITH THE GOVERNING LOCAL BUILDING CODE OR LOCAL JURISDICTIONAL REQUIREMENTS.

2. DESIGN LOADS

A. THE DESIGN DEAD LOADING FOR ALL FRAMING IS BASED ON THE CONSTRUCTION MATERIALS SHOWN ON THE DRAWINGS AND INDICATED IN THE SPECIFICATIONS. ALL FRAMING IS DESIGNED FOR THE FOLLOWING UNIFORM DEAD LOADS APPLIED IN ADDITION TO STRUCTURE SELF WEIGHT:

MECH., ELECT., PLUMBING, CEILING, ETC.....15PSF
ROOFING.....5PSF

B. THE MINIMUM DESIGN UNIFORMLY DISTRIBUTED LIVE LOADING FOR ALL NEW FRAMING SHALL BE AS FOLLOWS:

FLOOR LIVE LOADS
UNINHABITABLE ATTICS W/ STORAGE.....20PSF
SLEEPING AREAS.....30PSF
ALL OTHER AREAS INCL. STAIRS.....40PSF

ROOF LIVE LOAD.....20PSF

C. THE CONTRACTOR IS COMPLETELY RESPONSIBLE FOR THE METHOD OF CONSTRUCTION AND SHALL PROVIDE ALL TEMPORARY BRACING AND SHORING REQUIRED TO MAINTAIN THE STABILITY OF THE STRUCTURE AND TO SUPPORT CONSTRUCTION LOADS DURING CONSTRUCTION, INCLUDING SOILS ON WALLS FROM BACKFILLING PRIOR TO PLACING SLABS ON GRADE. DESIGN OF ALL BRACING IS THE CONTRACTORS RESPONSIBILITY. ANY SHORING OR BRACING ENGINEERING SHALL BE BY OTHERS.

3. MISCELLANEOUS

A. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY THE CONTRACTOR OR OWNER FOR REVIEW BY THE ENGINEER. IF THE CONTRACTOR OR OWNER FAILS TO SUBMIT THE SHOP DRAWINGS, THE ENGINEER WILL NOT BE RESPONSIBLE FOR STRUCTURAL CERTIFICATION AND DESIGN OF THE PROJECT.

B. THE CONTRACTOR SHALL REVIEW THE ARCHITECTURAL, MECHANICAL, ELECTRICAL PLUMBING AND STRUCTURAL DRAWINGS FOR LOCATION AND DIMENSION OF CHASES, INSERTS, OPENINGS, SLEEVES, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS.

C. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS SHOWN ON THE CONTRACT DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DISCREPANCIES AND OMISSIONS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN PROFESSIONALS.

D. SCALES SHOWN ON THE CONSTRUCTION DOCUMENTS ARE FOR GENERAL INFORMATION ONLY. DIMENSIONAL INFORMATION SHALL NOT BE OBTAINED BY SCALING THE DRAWINGS.

4. SPREAD FOOTING FOUNDATIONS

A. THE BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE LOCATED AS INDICATED ON THE DRAWINGS.

B. ALL FOOTINGS HAVE BEEN DESIGNED FOR AN ASSUMED NET ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. THE ALLOWABLE SOIL BEARING PRESSURE SHALL BE FIELD VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PLACING FOUNDATIONS. SHOULD THE ACTUAL SOIL BEARING PRESSURE BE LESS THAN 2000 PSF, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER.

C. ALL FILL PLACED UNDER SPREAD FOOTINGS SHALL BE COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698 OR ALTERNATIVE REQUIREMENTS PROVIDED BY A GEOTECHNICAL ENGINEER.

D. ALL EXISTING SOIL CONTAINING GRAVEL, CONSTRUCTION OR DEMOLITION DEBRIS, ORGANIC SUBSTANCES, OR OTHER FOREIGN OBJECTS SHALL BE REMOVED FROM THE REGION WITHIN THE FOOTPRINT OF THE STRUCTURE.

5. STRUCTURAL FILL

A. NEW FILL MATERIAL AND EXISTING BASE MATERIAL SHALL BE FREE OF ALL REFUSE, DEBRIS, AND ORGANIC MATTER AND SHALL BE APPROVED FOR USE BY A GEOTECHNICAL ENGINEER.

B. FILL MATERIAL SHALL BE DEPOSITED IN 8 INCH MAXIMUM LOOSE LIFTS AND COMPACTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D 698. FILL SHALL BE PLACED AND COMPACTED IN 8 INCH LOOSE LIFTS TO DESIRED FINISHED GRADE, OR ALTERNATIVE REQUIREMENTS PROVIDED BY A GEOTECHNICAL ENGINEER.

6. SLABS ON GRADE

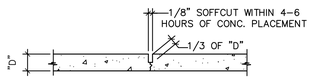
A. ALL SLABS ON GRADE SHALL CONSIST OF A 4 INCH THICK 2500 PSI CONCRETE SLAB PLACED OVER A 10 MIL (MIN.) POLYETHYLENE VAPOR RETARDER OVER CLEAN COMPACTED TERMITE TREATED FILL (RECRUIT 2 SYSTEM OR APPROVED EQUAL). REINFORCED WITH POLYPROPOLENE FIBERS (FIBERMESH OR EQUAL), MIN. 1–1/2" LONG AT A RATE OF 1.5 LBS/ CY OF CONCRETE. ALL EDGES OF VAPOR RETARDER SHALL BE LAPPED 6 INCHES AND TAPED.

B. SLABS ON GRADE SHALL BE SCREEDED, FLOATED, AND STEEL TROWELED TO FORM A SMOOTH, DENSE, AND PLANE SURFACE.

C. PLACE CONCRETE PER ACI 302. CONTRACTOR SHALL READ, UNDERSTAND FOLLOW GUIDELINES SET FORTH FOR PREPARING SUBGRADE, PLACING, CONSOLIDATING, FINISHING AND CURING CONCRETE SLABS.

D. EXTEND SLAB 1–1/2" INCHES AT DOOR OPENINGS, EXTEND SLAB 1–1/2" INCHES AT SLIDING GLASS DOORS AND RECESS ¾" FOR TRACKS.

E. SAWCUT CONTROL JOINTS IN CONCRETE SLAB RECOMMENDED AT 12'–0" O.C. MAX



NOTE: RECOMMENDED SPACING OF CONTROL JOINTS NOT TO EXCEED MAX. SPACING OF 12'–0"

7. CAST IN PLACE CONCRETE

A. ALL CONCRETE CONSTRUCTION SHALL CONFORM TO THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301)"; AND TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318)".

B. IN ADDITION TO THE ABOVE, ALL CONCRETE WORK SHALL CONFORM TO THE FOLLOWING:

1. REC. PRACTICE FOR HOT WEATHER CONCRETING (ACI 305).
2. REC. PRACTICE FOR COLD WEATHER CONCRETING (ACI 306).
3. REC. PRACTICE FOR CONCRETE FORMWORK (ACI 347).

C. ALL CONCRETE EXPOSED TO PUBLIC VIEW SHALL CONFORM TO THE REQUIREMENTS FOR ARCHITECTURAL CONCRETE CONTAINED IN ACI 301.

D. ALL CONCRETE; UNLESS NOTED OTHERWISE, SHALL BE STONE AGGREGATE CONCRETE HAVING THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:

FOUNDATIONS	3000 PSI
SLABS ON GRADE	3000 PSI
BEAMS/COLUMNS/ELEVATED SLABS	4000 PSI

ALL CONCRETE EXPOSED TO WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 5% +/– 1%. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. MAXIMUM AGGREGATE SIZE SHALL BE 1", AND MAXIMUM SLUMP SHALL BE 4", 3" FOR SLABS ON GRADE. ALL CONCRETE EXCEPT FOOTINGS SHALL CONTAIN A WATER REDUCING ADMIXTURE. PORTLAND CEMENT SHALL CONFORM TO ASTM C 150 AND NORMAL WEIGHT AGGREGATES SHALL CONFORM ASTM C 33. PORTLAND CEMENT SHALL CONFORM TO ASTM C 150 AND NORMAL WEIGHT AGGREGATES SHALL CONFORM TO ASTM C 33.

E. ALL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO GRADE 60 MIN.. ALL WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A 185. LAP ALL REINFORCING BARS A MINIMUM OF 48 BAR DIAMETERS AND ALL W.W.F. A MINIMUM OF TWO FULL GRIDS, UNLESS OTHERWISE INDICATED.

G. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE CRSI "MANUAL OF STANDARD PRACTICE", ACI 315" DETAILS AND DETAILING OF CONCRETE REINFORCEMENT", ACI SP 66 "DETAILING MANUAL".

H. GROUND BLAST FURNACE SLAG MAY BE USED TO REPLACE UP TO 50 PERCENT OF THE PORTLAND CEMENT IN A CONCRETE MIX, AND FLY ASH OR POZZOLAN MAY BE USED TO REPLACE UP TO 25 PERCENT OF PORTLAND CEMENT, SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND SHALL CONFORM TO ASTM C 989.

L. MINIMUM COVER FOR ALL REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE INDICATED:

FOUNDATIONS	3 INCHES
SLABS ON GRADE	2 INCHES (TOP)
ALL OTHER CONCRETE	ACI STANDARDS

8. CONCRETE MASONRY

A. ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530/ASCE 5/TMS 402)" AND TO THE "SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1/ASCE 6/TMS 602)".

B. ALL CONCRETE MASONRY SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH OF MASONRY, f'm = 1500 PSI.

C. ALL WALLS SHALL BE CONSTRUCTED OF NORMAL WEIGHT HOLLOW LOAD BEARING UNITS CONFORMING TO ASTM C 90 WITH A NET COMPRESSIVE STRENGTH OF 1900 PSI.

D. ALL EXTERIOR WALLS AND BEARING WALLS SHALL BE CONSTRUCTED WITH TYPE "S" CEMENT LIME MORTAR ABOVE GRADE, AND TYPE "M" BELOW GRADE. ALL MORTAR SHALL CONFORM TO ASTM C 270 WITH TYPE "S" MORTAR OBTAINING AN AVERAGE COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS AND TYPE "M" MORTAR OBTAINING AN AVERAGE COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS.

E. ALL FILL FOR CONCRETE MASONRY SHALL BE GROUT CONFORMING TO ASTM C 476 WITH A COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS.

F. ALL MASONRY WALLS SHALL BE REINFORCED WITH 9 GA TRUSS TYPE GALVANIZED HORIZONTAL JOINT REINFORCING CONFORMING TO ASTM A 82 AND SPACED VERTICALLY AT 16" O.C., UNLESS OTHERWISE INDICATED. PROVIDE CORNER AND TEE PIECES AT ALL INTERSECTIONS. LAP ALL JOINTS 6" MINIMUM.

G. ALL REINFORCING BARS FOR MASONRY SHALL BE NEW BILLET STEEL CONFORMING TO GRADE 60 MIN.

H. ALL VERTICAL REINFORCING SHALL BE LAP SPLICED A MINIMUM OF 25" UNLESS OTHERWISE INDICATED. ALL MASONRY CORES CONTAINING VERTICAL REINFORCING SHALL BE GROUTED SOLID.

I. IF VERTICAL REINF. IS MISSING/MISPLACED, REPLACEMENT REINF. MAY BE DOWELLED INTO FOOTING 5" MIN. WITH SIMPSON SET OR HP–ET EPOXY.

9. WOOD

A. ALL STRUCTURAL SHEATHING SHALL CONFORM TO THE DESIGN SPECIFICATIONS OF THE AMERICAN PLYWOOD ASSOCIATION. APA PS 1. PLYWOOD SHALL HAVE A MINIMUM OF THREE CONTINUOUS SPANS WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. ALL SHEATHING SHALL BE SPAN RATED FOR THE LOADS AND FRAMING SPACING AS INDICATED ON THE DRAWINGS.

B. FLOOR SHEATHING:
USE 25/32" PLYWOOD (MIN.) FOR FLOOR SHEATHING. FASTEN PLYWOOD TO FLOOR TRUSSES W/ #8 SCREWS AT 6" O.C. AND GLUE.

C. ROOF SHEATHING:
USE 1/2" CDX. OR OSB PLYWOOD (MIN.) FOR ROOF SHEATHING. FASTEN TO ROOF TRUSSES W/ 8d RING SHANK NAILS @ 6" O.C. AT ALL EXTERIOR SUPPORTS AND BLOCKING AND 6" O.C. AT ALL INTERIOR SUPPORTS. (MIN. PENETRATION 1–1/2") REDUCE NAIL SPACING TO 4" O.C. WITHIN 4'–0" OF ALL ROOF EDGES PER DIAGRAM SHOWN BELOW (ZONES 2 AND 3).

D. VERTICAL WALL SHEATHING (GABLES, TRUSS HEELS ETC.):
USE 15/32" CDX. OR 7/16" OSB PLYWOOD (MIN.) FOR WALL SHEATHING. FASTEN PLYWOOD TO STUDS W/ 8d NAILS @ 6" O.C. AT ALL EXTERIOR SUPPORTS AND BLOCKING AND 6" O.C. AT ALL INTERIOR SUPPORTS. (MIN. PENETRATION 1–1/2")

E. WOOD GRADE SHALL BE REGULAR SOUTHERN PINE NO. 2, 19% MOISTURE CONTENT MAX., Fb = 1400psi MIN. 2x4 STUDS MAY BE SPRUCE OR PINE.

F. ALL MANUFACTURED/GLULAM/POWERBEAM/LVL MEMBERS TO HAVE A MINIMUM BENDING STRESS OF Fb = 2250psi.

G. ALL DOUBLE BEAMS TO BE CONNECTED WITH 12d NAILS AT 16" O.C. STAGGERED, MIN. ALL TRIPLE AND QUADRUPLE BEAMS TO BE CONNECTED WITH ⅝" DIA. LAG SCREWS AT 16" O.C. ON BOTH FACES OF BEAM.

H. ALL SIMPSON CONNECTORS TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS, INCLUDING NUMBER, LENGTH AND DIAMETER OF FASTENER. WHERE CONNECTOR HAS MULTIPLE OPTIONS FOR NUMBER OF NAIL HOLES TO BE FILLED, THE MAX. NUMBER OF NAIL HOLES SHOULD BE FILLED U.N.O.

I. ALL WOOD IN CONTACT WITH THE GROUND AND THAT SUPPORTS PERMANENT STRUCTURES INTENDED FOR HUMAN OCCUPANCY SHALL BE APPROVED PRESERVATIVE TREATED WOOD FBC R317.1.2.

J. INTERIOR NON–LOAD BEARING STUD WALLS TO BE 2x4 STUDS SPACED AT 24" ON CENTER.

10. PRE–ENGINEERED WOOD ROOF TRUSSES

A. WOOD TRUSSES SHALL DESIGNED FOR THE LOADS INDICATED ON THE CONTRACT DOCUMENTS. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED AND SPECIFIED BY THE TRUSS SUPPLIER.

B. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS FOR ALL WOOD FLOOR TRUSSES INCLUDING TRUSS LAYOUT, WOOD SPECIES AND GRADE, MEMBER SIZES, TRUSS BEARING CONNECTION DETAILS WITH CLEARLY INDICATED REACTIONS, AND THE DESIGN AND LOCATION OF ALL REQUIRED BRACING AND BRIDGING. THE TRUSSES SHALL BE DESIGNED IN A MANNER THAT AVOIDS ADDITIONAL MEMBER BRACING. THE CALCULATIONS AND SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA.

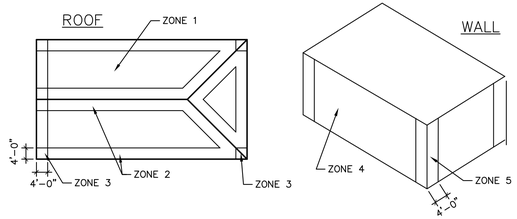
11. WIND LOADING:

DESIGN WIND SPEED Vult = 139 MPH (Vasd = 108 MPH)
RISK CATEGORY = II
WIND EXPOSURE CATEGORY = C
INTERNAL PRESSURE COEFF. (ASCE ENCLOSED BLDG) = ± 0.18

COMPONENTS AND CLADDING
WIND PRESSURE SCHEDULE
(ROOF ANGLE 7° – 27°)

	ZONE	TRIBUTARY AREA	PRESSURE
ROOF	1	10 20 50 100	+24.6, –39.1 +22.4, –38.0 +19.5, –36.5 +17.3, –35.5
	2	10 20 50 100	+24.6, –68.0 +22.4, –62.6 +19.5, –55.3 +17.3, –49.9
	3	10 20 50 100	+24.6, –100.6 +22.4, –94.0 +19.5, –85.3 +17.3, –78.8
WALL	4	10 20 50 100	+42.7, –46.2 +40.8, –44.4 +38.2, –41.9 +36.3, –39.9
	5	10 20 50 100	+42.7, –57.1 +40.8, –53.2 +38.2, –48.2 +36.3, –44.4

ALL WINDOWS AND DOORS TO BE SELECTED FROM WALL PRESSURE TABLES. VALUES ARE ULTIMATE, FOR ALLOWABLE LOADS MULTIPLY LISTED VALUES BY 0.6. ZONES 2,3&5 WITHIN 8'–0" OF CORNERS



1 S-XX STRUCTURAL GENERAL NOTES
SCALE: N.T.S.

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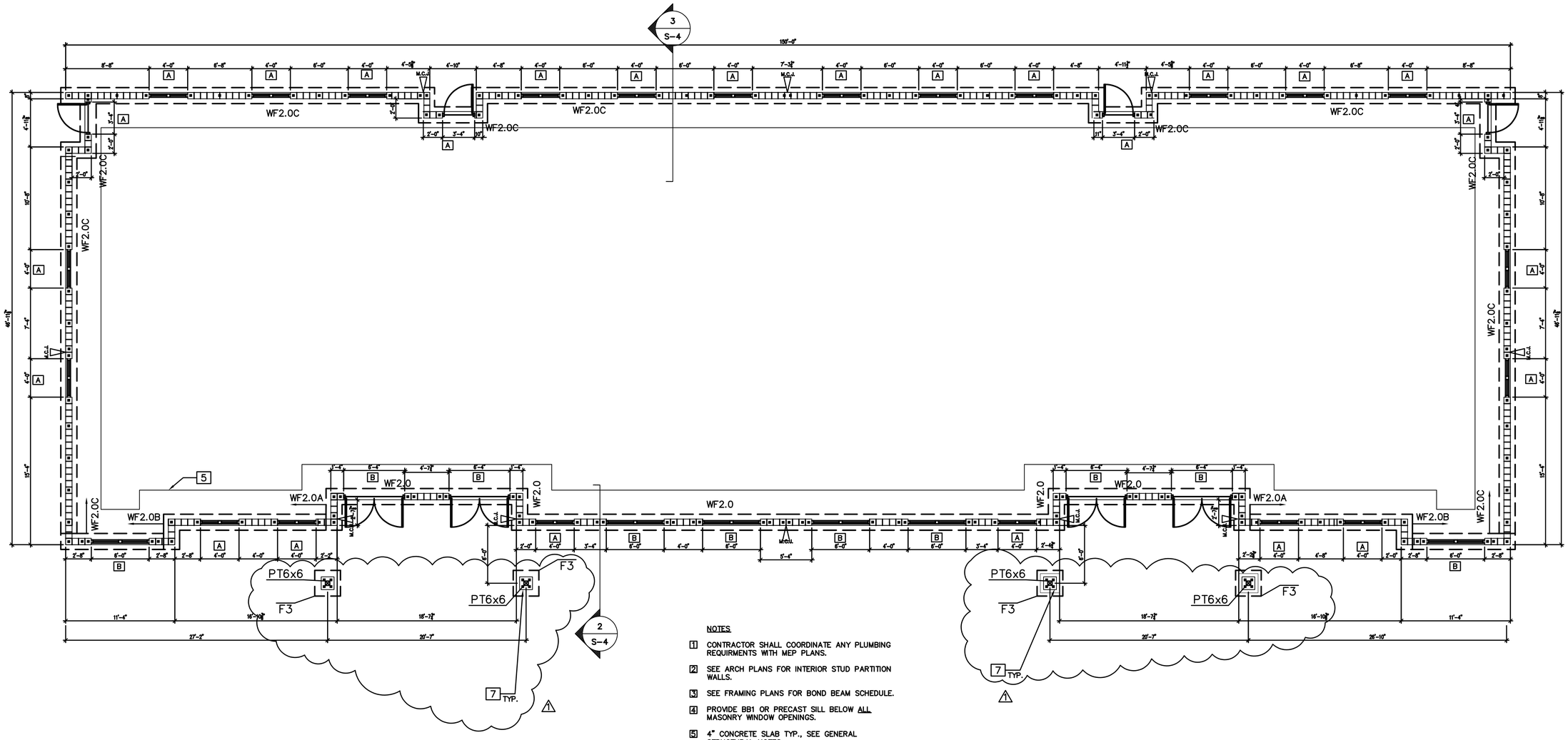
SEAL

A PROPOSED DARK SHELL FOR:
BONNEVILLE DR PROP OFFICE BLG.
1716, 1718, AND 1720 BONNEVILLE DR
ORLANDO, FLORIDA

DATE:	01/22/2018
PROJECT:	160202
DRAWN BY:	DS
CHECKED BY:	DS
SHEET	OF

S–1



NOTES

1. CONTRACTOR SHALL COORDINATE ANY PLUMBING REQUIRMENTS WITH MEP PLANS.
2. SEE ARCH PLANS FOR INTERIOR STUD PARTITION WALLS.
3. SEE FRAMING PLANS FOR BOND BEAM SCHEDULE.
4. PROVIDE BB1 OR PRECAST SILL BELOW ALL MASONRY WINDOW OPENINGS.
5. 4" CONCRETE SLAB TYP., SEE GENERAL STRUCTURAL NOTES.
6. PROVIDE CONTROL JOINTS IN FLOOR SLAB AT 12'-0" O.C. MAX. (SEE GEN. NOTES SHEET), COORD. WITH ARCH PLANS FOR FLOOR FINISHES.
7. SIMPSON ABU66 POST BASE CONNECTOR INTO TOP OF CMU PIER. CMU PIER 16"x16" WITH (4) #5 VERT. REINFORCING SPACED AT 12 O.C.

PROVIDE 8" CMU WALL WITH #5 REINFORCING IN CENTER OF SOLID GROUTED CELLS AT SPACING SHOWN, AT 40" O.C. MAX AND ALL CORNERS, JAMBS AND MASONRY CONTROL JOINTS..

M.C.J. MASONRY CONTROL JOINTS SHALL BE PLACED ON OUTSIDE EDGES OF ANY FILLED CELLS OR JAMBS AND SHALL NOT CUT THROUGH ANY BOND BEAMS OR OTHER REINFORCED, SOLID GROUTED CELLS

FIELD VERIFY ALL DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO BEGINNING WORK

FOUNDATION SCHEDULE

MARK	SIZE	DEPTH	REINFORCEMENT	T.O. FTG. EL.
WF2.0	1'-8" CONT.	1'-0"	3- #5 CONT. BOTTOM	-1'-4"
WF2.0A	1'-8" CONT.	1'-0"	3- #5 CONT. BOTTOM	-2'-8"
WF2.0B	1'-8" CONT.	1'-0"	3- #5 CONT. BOTTOM	-4'-0"
WF2.0C	1'-8" CONT.	1'-0"	3- #5 CONT. BOTTOM	-5'-4"
F3	2'-8"x2'-8"	1'-0"	(4) #5s E/W BOTTOM	-1'-4"

BOND BEAM/LINTEL SCHEDULE

MARK	BEAM	ELEVATION	LOCATION
A	8" PRECAST LINTEL	(SEE ARCH ELEV.)	OPENINGS LESS THAN 4'-8"
B	16" PRECAST LINTEL	(SEE ARCH ELEV.)	OPENINGS LESS THAN 8'-0"
C	24" PRECAST LINTEL	(SEE ARCH ELEV.)	ALL OTHER OPENINGS
SEE DETAIL SHEETS FOR BOND BEAM/LINTEL DETAILS			

1 S-2 FOUNDATION PLAN
SCALE: 3/16" = 1'-0"

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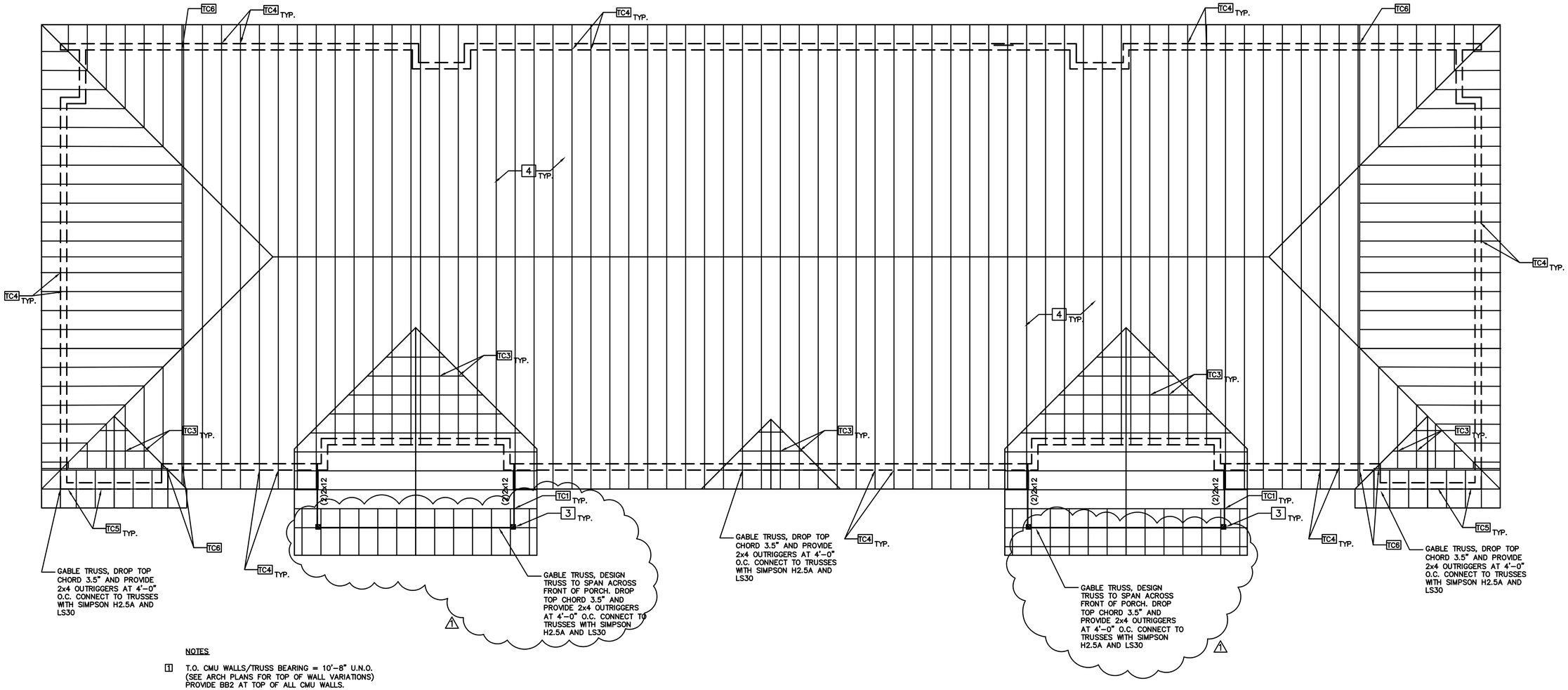
S-2



NOTES

1. T.O. CMU WALLS/TRUSS BEARING = 10'-8" U.N.O. (SEE ARCH PLANS FOR TOP OF WALL VARIATIONS) PROVIDE BB2 AT TOP OF ALL CMU WALLS.
2. T.O. WOOD BEAM ELEVATIONS = SEE ARCH PLANS
3. CONNECT ALL WOOD BEAM TO WOOD POSTS WITH (2) SIMPSON LSTA15 STRAPS FRONT AND (2) SIMPSON MTS16 STRAPS REAR. CONNECT TO CMU WALLS WITH SIMPSON HTSM20 PROVIDE BLOCKING AS NECESSARY.
4. SEE GENERAL STRUCTURAL NOTES FOR PLYWOOD SHEATHING

TRUSS CONNECTOR SCHEDULE

CALLOUT	CONNECTOR	ALLOWABLE UPLIFT LOAD
TC1	MTS12	1000 LB
TC2	LSTA15 @ 48" O.C.	1110 LB
TC3	VTCR @ 48" O.C.	1245 LB
TC4	HETA16	1850 LB
TC5	HETA16 @ 48" O.C.	1850 LB
TC6	MGT	3950 LB

1 S-3 ROOF FRAMING PLAN
SCALE: 3/16" = 1'-0"

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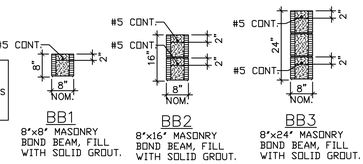
SHEET OF

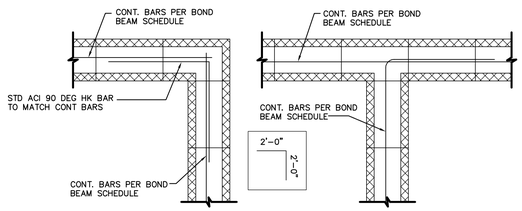
S-3

BOND BEAM REINFORCEMENT:

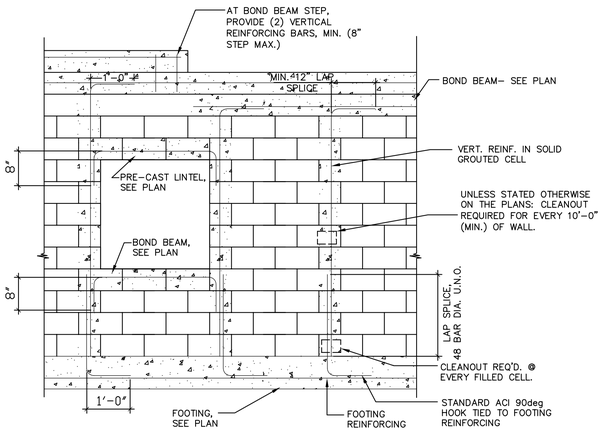
LAP ALL HORIZ. BARS 30" MIN. USE ACI 90 DEG. BENDS AT ALL CORNERS. ALL PIERS, BOND BEAMS AND LINTELS SHALL BE FILLED SOLID W/ 3000psi GROUT



BOND BEAM DETAILS

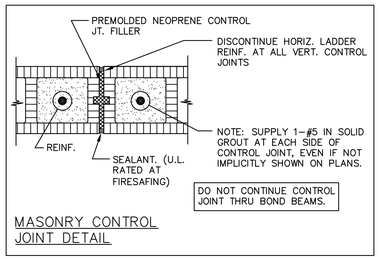


BOND BEAM CORNER DETAILS



TYPICAL CMU WALL CONSTRUCTION DETAIL

GROUT SPACE REQUIREMENTS (FINE GROUT)	
MAXIMUM GROUT POUR HEIGHT, FT	MINIMUM GROUT SPACE DIMENSIONS FOR GROUTING CELLS OF HOLLOW UNITS, IN. x IN.
1	1-1/2 x 2
5	2 x 3
12	2-1/2 x 3
24	3 x 2



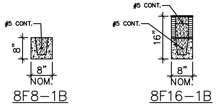
MASONRY CONTROL JOINT DETAIL

LINTEL MARK	CAST-CRETE 8" LENGTH[#0]	TYPE	GRAVITY 8F8-1B	UPLIFT 8F8-1T	LATERAL 8F8
"A"	2'-10"	PRECAST	3069	1972	1642
	3'-6"	PRECAST	3069	1569	1024
	4'-0"	PRECAST	2693	1363	763
	4'-6"	PRECAST	2189	1207	591
	5'-4"	PRECAST	1663	1016	411
	5'-10"	PRECAST	1451	909	339
	6'-6"	PRECAST	1238	835	721
	7'-6"	PRECAST	1011	727	534
	9'-4"	PRECAST	752	591	512
	10'-6"	PRECAST	643	530	401
	11'-4"	PRECAST	582	474	452
	12'-0"	PRECAST	540	470	402
	13'-4"	PRECAST	471	418	452
	14'-0"	PRECAST	442	384	324
	14'-8"	PRESTRESSED	458	239	293
	15'-4"	PRESTRESSED	412	224	284
	17'-4"	PRESTRESSED	300	187	259
	19'-4"	PRESTRESSED	235	162	194
	21'-4"	PRESTRESSED	180	142	1972

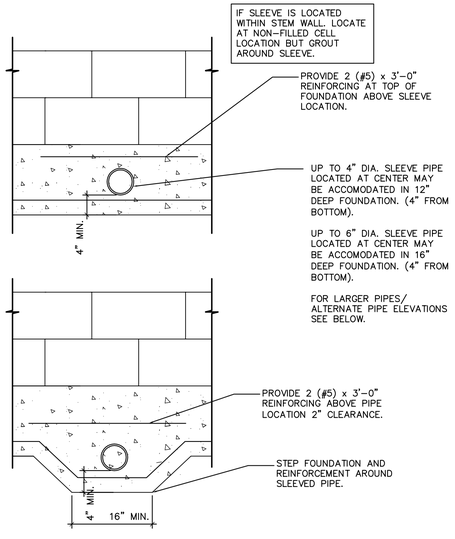
LINTEL MARK	CAST-CRETE 16" LENGTH[#0]	TYPE	GRAVITY 8F16-1B	UPLIFT 8F16-1T	LATERAL 8F8
"B"	2'-10"	PRECAST	6113	4460	1642
	3'-6"	PRECAST	6113	3547	1024
	4'-0"	PRECAST	6113	3079	763
	4'-6"	PRECAST	6113	2724	591
	5'-4"	PRECAST	5365	2290	411
	5'-10"	PRECAST	4360	2093	339
	6'-6"	PRECAST	3480	1880	721
	7'-6"	PRECAST	2661	1634	534
	9'-4"	PRECAST	1843	1133	512
	10'-6"	PRECAST	1533	914	401
	11'-4"	PRECAST	1366	798	452
	12'-0"	PRECAST	1254	723	402
	13'-4"	PRECAST	1075	606	452
	14'-0"	PRECAST	1002	559	324
	14'-8"	PRESTRESSED	1370	519	293
	15'-4"	PRESTRESSED	1250	485	284
	17'-4"	PRESTRESSED	950	404	259
	19'-4"	PRESTRESSED	750	347	194
	21'-4"	PRESTRESSED	598	306	1972

PRECAST LINTEL NOTES:

1. ALL LINTEL DETAILS AND LOAD TABLES, WERE REPRODUCED FROM THE "SAFE LOAD TABLES, HIGH STENGTH PRECAST & PRESTRESSED CONCRETE LINTELS" MANUAL PUBLISHED BY CAST-CRETE. THEY ARE REPRODUCED HERE FOR INFORMATIONAL PURPOSES ONLY. ALL ACTUAL LINTEL DESIGN, DETAILS, AND ENGINEERING BY CAST-CRETE. STRUCTURAL ENGINEERING HAS SELECTED THE LINTELS BASED ON THE SERVICE LOADS FOR THIS PROJECT.
2. PROVIDE MIN. 4" BEARING @ EACH END, 8" PREFERRED.
3. TURN BARS DOWN INTO FILLED CELLS AND LAP 8" MIN. W/ VERT BARS..
4. BARS CALLED OUT ON DETAILS ARE ADDITIONAL BARS ADDED IN FIELD. PLACE AS SHOWN.
5. LENGTH LISTED IN TABLE INDICATE TOTAL LENGTH, DEDUCT 8" FOR ACTUAL CLEAR SPAN OF OPENING. (EX. 5'-4" LENGTH = 4'-8" CLEAR SPAN)
6. CONTRACTOR MAY SUBSTITUTE AN ALTERNATIVE LINTEL MANUFACTURER PROVIDED THAT THE ALLOWABLE LOADS MEET OR EXCEED THOSE SHOWN IN THESE TABLES.



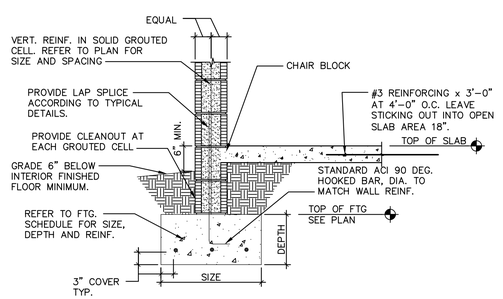
PRECAST LINTEL DETAILS AND LOAD TABLES



SLEEVED FOUNDATION DETAIL

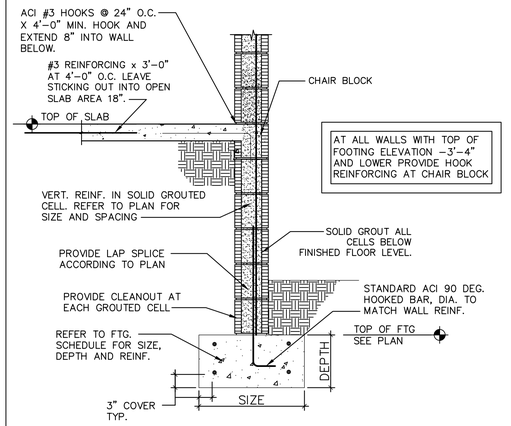
1 S-4 TYPICAL CMU DETAILS

SCALE: NTS



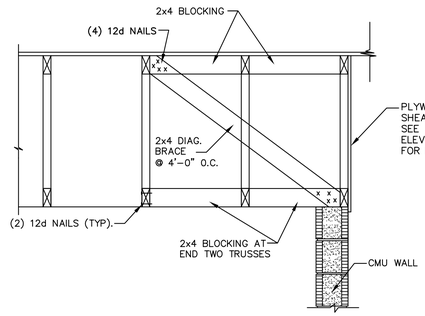
2 S-4 WALL FTG DETAIL (FRONT)

SCALE: NTS



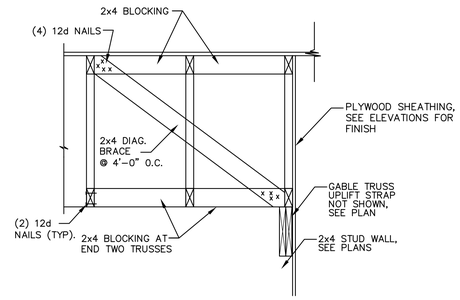
3 S-4 WALL FTG DETAIL (REAR)

SCALE: NTS



4 S-4 GABLE DETAIL (CMU)

SCALE: NTS



5 S-4 GABLE DETAIL (WOOD FRAMING)

SCALE: NTS

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PHONE: (407) 421-4866

SEAL

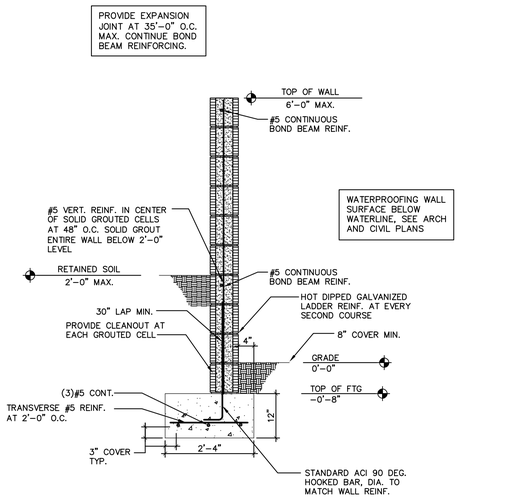
A PROPOSED DARK SHELL FOR:

BONNEVILLE DR PROP OFFICE BLG.

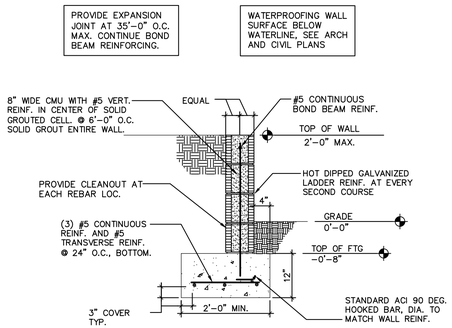
1716, 1718, AND 1720 BONNEVILLE DR
ORLANDO, FLORIDA

DATE:	01/22/2018
PROJECT:	160202
DRAWN BY:	DS
CHECKED BY:	DS
SHEET	OF

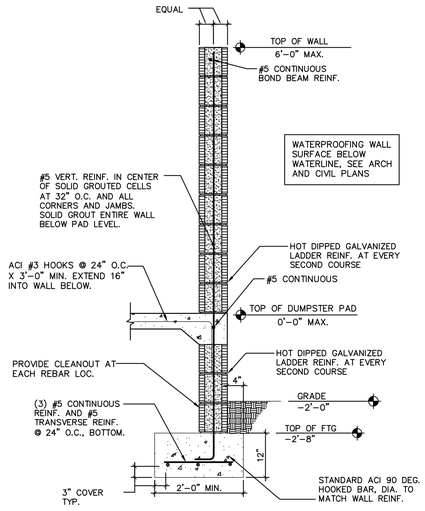
S-4



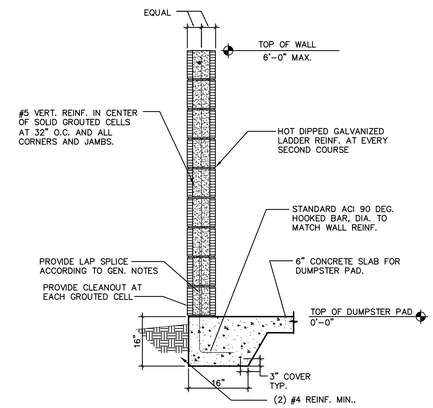
1 S-5 2'-0" RETAINING WALL WITH 4'-0" SCREEN WALL
SCALE: NTS



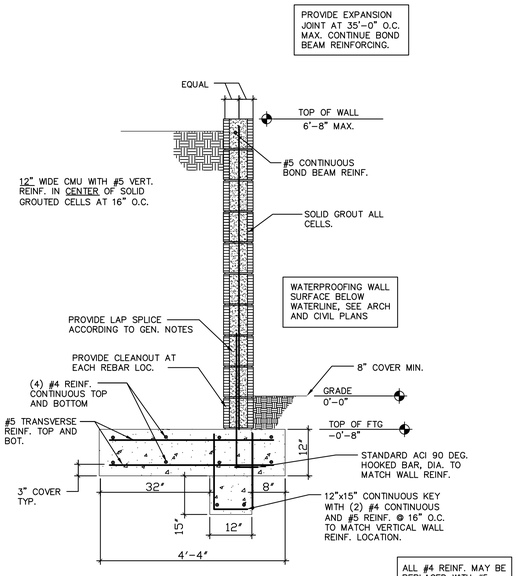
2 S-5 2'-0" RETAINING WALL DETAIL
SCALE: NTS



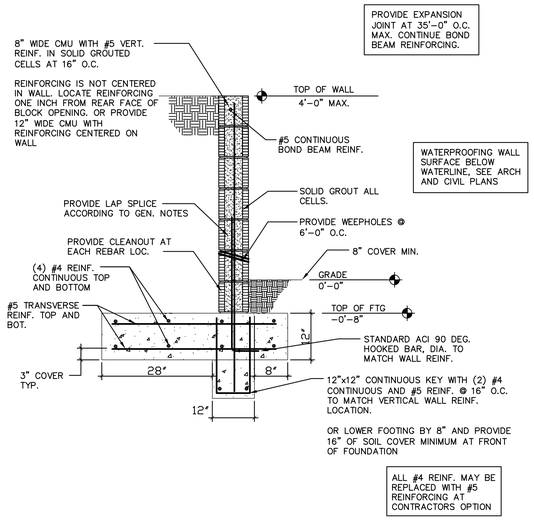
3 S-5 DUMPSTER SCREEN WALL WITH 2'-0" RET.
SCALE: NTS



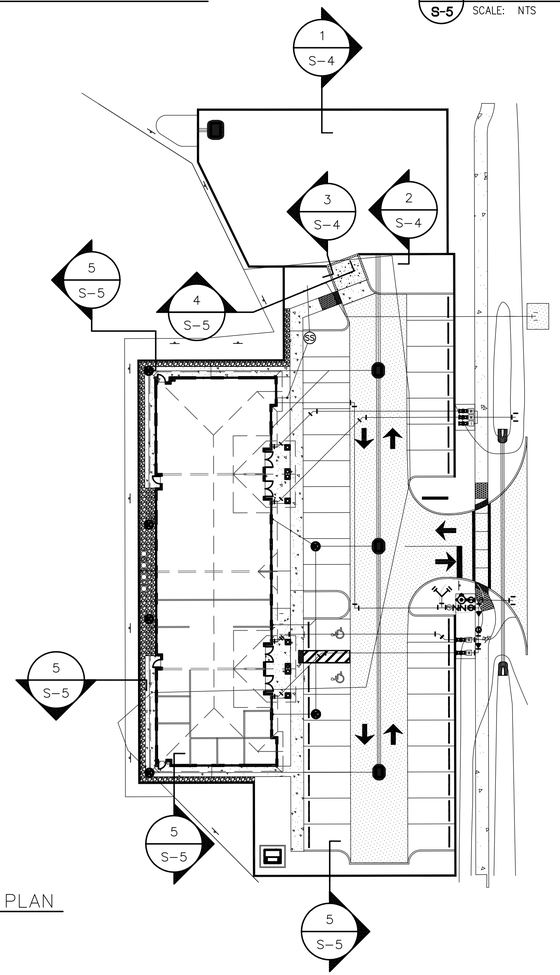
4 S-5 DUMPSTER SCREEN WALL
SCALE: NTS



5 S-5 6'-0" MAX. RETAINING WALL DETAIL
SCALE: NTS



5a S-5 4'-0" MAX. RETAINING WALL DETAIL
SCALE: NTS



6 S-5 SITE WALL KEY PLAN
SCALE: NTS

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